Effect of Using Plastic Waste on Mechanical Properties of Concrete

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ABSTRACT

The present study deals with the effect of using plastic waste on mechanical properties of concrete. The experimental work consists of adding manually formed serrated plastic fibers by ratios of (1%, 2% and 3%) of cement weight. Different lengths of fibers were used for each ratio with a constant width of 5 mm. It was noticed from laboratory tests that the addition of plastic fibers improves the compressive strength, tensile strength and modulus of rupture of concrete, by different ratios, about 20%, furthermore it could be a sufficient method to get rid of plastic waste.

Keywords: Recycle Plastic Waste, Mechanical Properties, Concrete, Compressive strength, Flexural Strength.

1. INTRODUCTION

Concrete is one of the most important materials used in construction, and because of its suitable cost and strength, an improvement of its mechanical and chemical properties was searched by many researchers by different processes, like adding different types polymers as a ratio of water content, or adding high strength steel fibers as a ratio of cement content. Some researches investigate the effect of adding low cost materials on the mechanical properties of concrete, and because plastic material is one of the environmental pollutants, it had been used in concrete mix in some researches.

Isamil and Al-Hashimi (2008)¹ studied involved 86 experiments and 254 tests to determine the efficiency of reusing waste plastic in the production of concrete. Thirty kilograms of waste plastic of fabriform shapes was used as a partial replacement for sand by 0%, 10%, 15%, and 20% with 800kg of concrete mixtures. All of the concrete mixtures were tested at room temperature. These tests include performing slump, fresh density, dry density, compressive strength, flexural strength, and toughness indices. Seventy cubes were molded for compressive strength and dry density tests, and 54 prisms were cast for flexural strength and toughness indices tests. Curing ages of 3,7,14, and 28days for the concrete mixtures were applied in this work. The results proved the arrest of the propagation of microcracks by introducing waste plastic of fabriform shapes to concrete mixtures. This study insures that reusing waste plastic as a sand-substitution aggregate in concrete gives a good approach to reduce the cost of materials and solve some of the solid waste problems posed by plastics.

Rai, et. al.(2012)¹² they worked om a number of concrete mixes where sand partially replaced by waste plastic flakes with varying percentages by volume of sand. Waste plastic mix concrete with and without superplasticizer. Forty-eight cube samples were moulded for compressive strength tests at three, seven, and twenty-eight days. Eight beams were also cast to study the flexural strength characteristic of waste plastic mix concrete. It was found that the reduction in workability and compressive strength, due to partially replacement of sand by waste plastic, is minimal and can be enhanced by addition of superplasticizer.

AL-Hadithi and Al-Ani (2015)² investigated the change in mechanical properties of High Performance Concrete (HPC) with added waste plastics in concrete. For this purpose, 2.5%, 5% and 7.5% in volume of natural fine aggregate in the HPC mixes were replaced by an equal volume of Polyethylene Terephthalate (PET) waste, got by shredded PET bottles. The mechanical properties (compressive, splitting tensile, and flexural strength) evaluated at the ages of (7,28,56 and 91) days while the static modulus of elasticity tested at (28 and 91) days. The results indicated that HPC containing PET-aggregate presented lower compressive strength and static elasticity. The splitting strength displayed an arising trend at the initial stages, however, they have a tendency to decrease after a while. On the other hand, flexural strength results gave better modulus of rapture at all ages of curing, as compared with reference concrete specimens.

In the present study, the plastic wastes were used as fibers with fixed width of 5 mm and three different lengths (10 mm, 20 mm and 30 mm) by ratios of (1%, 2% and 3%) of cement weight. The properties of the local used plastic fibers as well as concrete components were measured and compared with Iraqi specifications. The aim of the present research is to verify the effect of using these wastes in concrete and finding the best method to use it in improving concrete properties.

2. EXPERIMENTAL WORK

In the experimental work, 180 cubes and 30 cylinders (100×200)mm were casted and tested in the laboratory to find the compressive and indirect tensile strength on concrete, moreover 30 prisms of dimensions ($100\times100\times500$) mm were casted and used for finding the modulus of rupture for different waste ratios. Each type of mixing had 12 cubes of dimensions ($150\times150\times150$) mm and 6 cylinders of diameter 100 mm and height 200 mm. All used materials were tested and compared with Iraqi specifications. The tested materials are:

2.1 CEMENT

Al-Muthanna ordinary Portland cement was used in the experimental work. Chemical and physical tests were carried out in laboratory of Engineering collage/ University of Al-Qadisiyah. Test results are shown in Table (1).

Table (1): Results of Cement Test

Chemical Test Results								
Oxide	Percent weight	Iraqi Specification						
SiO_2	21.3	-						
CaO	64.64	-						
MgO	2.62	≤5%						
Fe ₂ O ₃	2.58	-						
Al_2O_3	6.16	-						
Loss on ignition	3	≤4%						
Irresolvable material	1.5	≤1.5%						
	Physical Test Results							
Property	Results	Iraqi Specification						
Soundness (Autoclave)%	0.02	≤0.8						
Initial setting time (min)	85	≥45						
Final setting time (min)	260	≤600						
Cor	npressive strength (MPa))						
3 days	16.3	≥15						
7 days	23.7	≥23						

2.2 FINE AGGREGATE

Washed fine Aggregate was used in the mix and it was tested according to Iraqi specification No. 45. The test results showed that it within zone 2 as shown in Table (2).

Table (2): Results of Fine Aggregate Test

Sieve size	% of pass	Iraqi specification
4.75 (mm)	100	90-100
2.36 (mm)	86	75-100
1.18 (mm)	68.6	55-90
600 micron	37.4	35-59
300 micron	8	8-30
150 micron	2.4	0-10
SO ₃ salt	Result	Iraqi specification
SO ₃ (%)	0.17	≤0.5%

2.3 COARSE AGGREGATE

Shattered coarse Aggregate was used in the mix also it was tested according to Iraqi specification No. 45. The test results are shown in Table (3).

Table (3): Results of Coarse Aggregate Test

Sieve size	% of pass	Iraqi specification
37.5 (mm)	100	100
20 (mm)	100	75-100
12.5 (mm)	88.48	-
9.5 (mm)	33.56	30-6
4.75 (mm)	0.6	0-10
2.36 (mm)	0.08	-
SO ₃ salt	Result	Iraqi specification
SO ₃ (%)	0.08	≤0.1%

2.4 PLASTIC FIBERS

Plastic fibers were manually formed from water and soft drinks bottles. These fibers were formed with a width of 5 mm and three different lengths (10, 20 and 30) mm, as shown in Figures (1, 2).

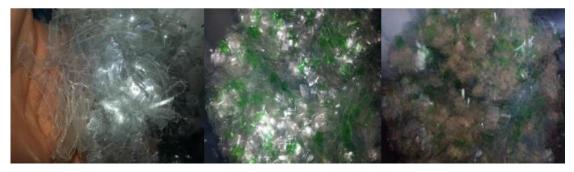


Figure (1): Formed Plastic Fibers

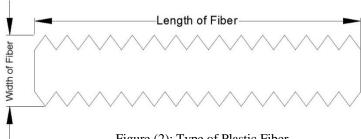


Figure (2): Type of Plastic Fiber

The density of these fibers was found equal to 1.01 gm/cm³ by using Archimedes theory. Also, elongation, yield stress and ultimate stress was found by testing a sample of plastic fibers using tensile test machine. The dimensions of the tested samples were as follows:

Total length of sample = 200 mm

Effective length = 30 mm

Effective width = 35 mm

Thickness of sample $\approx 0.6 \text{ mm}$

The results of elongation, yield stress and ultimate stress were 8 mm, 7.15 kN and 8.25 kN respectively.

3. CASTING METHOD

Mixing process was carried out by adding cement powder to fine and coarse aggregate with a mix ratio of (1:2:4) respectively. The mixture was blending in dry state for a minute to insure the homogeneity of the mixture, then the plastic fibers were added gradually during dry mixing process to avoid agglomeration of materials. Finally, the specified weight of water which is equal to 68% of cement weight was added to complete the casting process.

4. LABORATORY TESTS

The following laboratory tests were carried out in University of Al-Qadisiyah / College of Engineering. These tests show the effect of additive plastic fibers on mechanical properties of concrete.

4.1 SLUMP TEST

The base of examining workability of concrete containing plastic fibers is the slump test. Slump was measured for reference mixture which has no plastic fibers and that which contains different ratios of plastic fibers. It could be noticed that adding 1% of plastic fibers of lengths (10 mm, 20 mm and 30 mm) had no significant effect on mix workability, while the ratio of 2% shows little improvement in mix workability as shown Table (4).

Table (4): Slump Tests Results

w/c ratio	Fiber Length	Slump in (mm)								
w/c ratio	Fiber Length	Fiber ratio (1%)	Fiber ratio (2%)	Fiber ratio (3%)						
	Non	119	119	119						
0.60	10 mm	120	120	118						
0.68	20 mm	120	121	116						
	30 mm	117	116	114						

4.2 DENSITY TEST

The density of natural concrete with no fibers and those which contains fibers were measured and listed in Table (5). The results show that there is a little difference in density of concrete contains plastic fibers. This is because plastic fibers have very low density if compared with other concrete component and this difference could be neglected.

Table (5), Arrange	Management Dansit	ri for All Took	d Consimons	(1-a/m ³)
Table (5): Average	Measured Densit	y ioi Aii 1esu	ed Specimens	(Kg/III)

	Density For 10mm Fiber			Density For 20mm			Density For 30mm		
Referencemix	Length			Fiber Length			Fiber Length		
	1%	2%	3%	1%	2%	3%	1%	2%	3%
2401	2374	2357	2362	2414	2391	2429	2381	2456	2336

4.3 COMPRESSIVE STRENGTH TEST

The compressive strengths of all $(150 \times 150 \times 150)$ mm concrete cubes were found and recorded. The average value of compressive strength for ages (7 and 28) days was calculated and listed in Table (6). The obtained results of all specimens indicate an improvement of compressive strength in earlier age by about 40%, while in 28 day age the best result was obtained was for 20 mm fiber length at ratio of (2%) and was about 15%. The other ratios show little improvement in compressive strength and some of those gave values less than the reference cubes as shown in Figure (3:a and 3:b).

Table (6): Average Compressive Strength for All Tested Specimens (MPa)

Age	Ref. Mix	1	ssive Strei m Fiber L	C	Compressive Strength For 20 mm Fiber Length			Compressive Strength For 30 mm Fiber Length		
	0%	1%	2%	3%	1%	2%	3%	1%	2%	3%
7 days	13.98	15.25	16.49	15.86	17.9	19.34	17.9	16.3	17.37	15.94
28 days	20.93	21.5	22.8	21.29	21.7	23.07	20.5	21.1	21.72	19.1

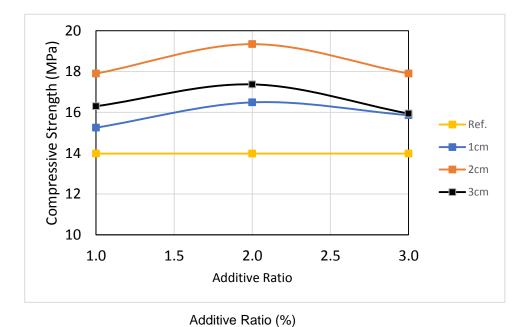


Figure 3: (a) Compressive Strength Curves for Cubes at (7) Day Age

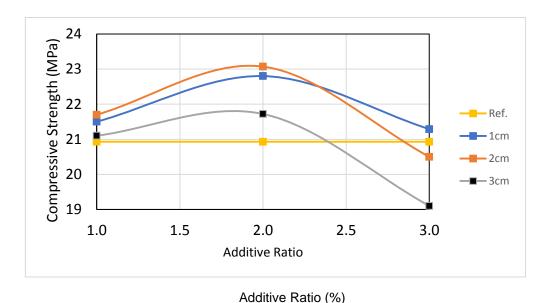


Figure 3: (b) Compressive Strength Curves for Cubes at (28) Day Age

4.4 TENSILE STRENGTH TEST

The indirect tensile strength of all (100mm diameter and 200 mm height) concrete cylinders were found and recorded. The average value of tensile strength for ages (7 and 28) days was also calculated and listed in Table (7). The obtained results of all specimens show an improvement of tensile strength in earlier age by about 40%, while in 28 day age the best result was obtained for 20 mm fiber length at ratio of (2%) which was about 35%. The other ratios show less improvement in tensile strength shown in Figure (4:a and 4:b).

Table (7): Average Compressive Strength for All Tested Specimens (MPa)

Age	Ref. Mix			gth For Length		Strength Fiber Le		Tensile Strength For 30 mm Fiber Length		
	0%	1%	2%	3%	1%	2%	3%	1%	2%	3%
7 days	1.57	2.18	2.19	2.22	1.99	2.1	2.1	1.9	2.08	2.0
28 days	2.13	2.5	2.64	2.53	2.57	2.89	2.67	2.43	2.55	2.46

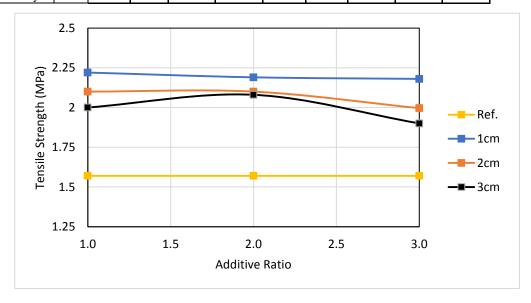
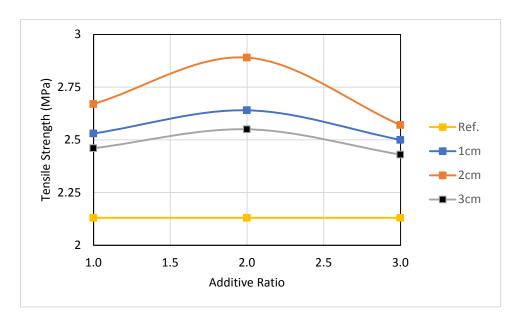


Figure 4: (a) Tensile Strength Curves for Cylinders at (7) Day Age



Additive Ratio (%)

Figure 4: (b) Tensile Strength Curves for Cylinders at (28) Day Age

4.5 MODULUS OF RUPTURE

The direct tensile strength for concrete prisms of dimensions $(100 \times 100 \times 500)$ mm under the action of two point loads, Figure 5, were found and recorded. The average value of tensile strength for 28 days age was calculated and shown in Figure 6. The obtained results for all prisms show an improvement of tensile strength, spatially in additive ratio of 2% with fiber length 20 mm the improvement was about 7%. The other ratios show less improvement in tensile strength.



Figure 5: Modulus of Rupture Test

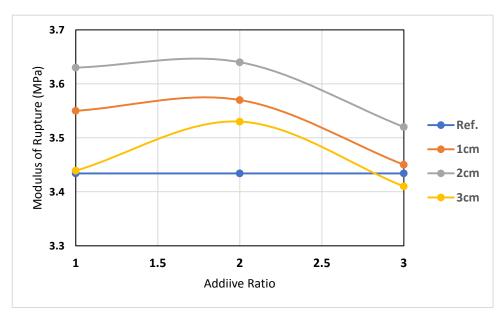


Figure 5: Modulus of Rupture results at (28) Day Age

5. CONCLUSIONS

After studying the previous results obtained from the laboratory tests, the following conclusions were found:

- In order to get rid of the large amount of plastic wastes, it is useful to be used in ordinary concrete structures with some improvement for its mechanical properties when these plastic fibers formed as serrated fiber.
- The addition of plastic fibers to concrete mix improve some mechanical properties of the concrete such as modulus of rupture, indirect tensile strength and compressive strength.
- The best ratio of adding plastic fibers for concrete to get maximum compressive strength and acceptable increase in tensile strength is (2%) with length of 20 mm and width of 5mm.
- For structures with high tensile stresses, the best ratio of adding plastic fibers for concrete is (2%) with length of 20 mm and width of 5mm.
- Also some reduction in concrete weight occurs due to additive of plastic fibers.

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